

Fully integrated load analysis included in the structural reliability assessment of a monopile supported offshore wind turbine

Authors

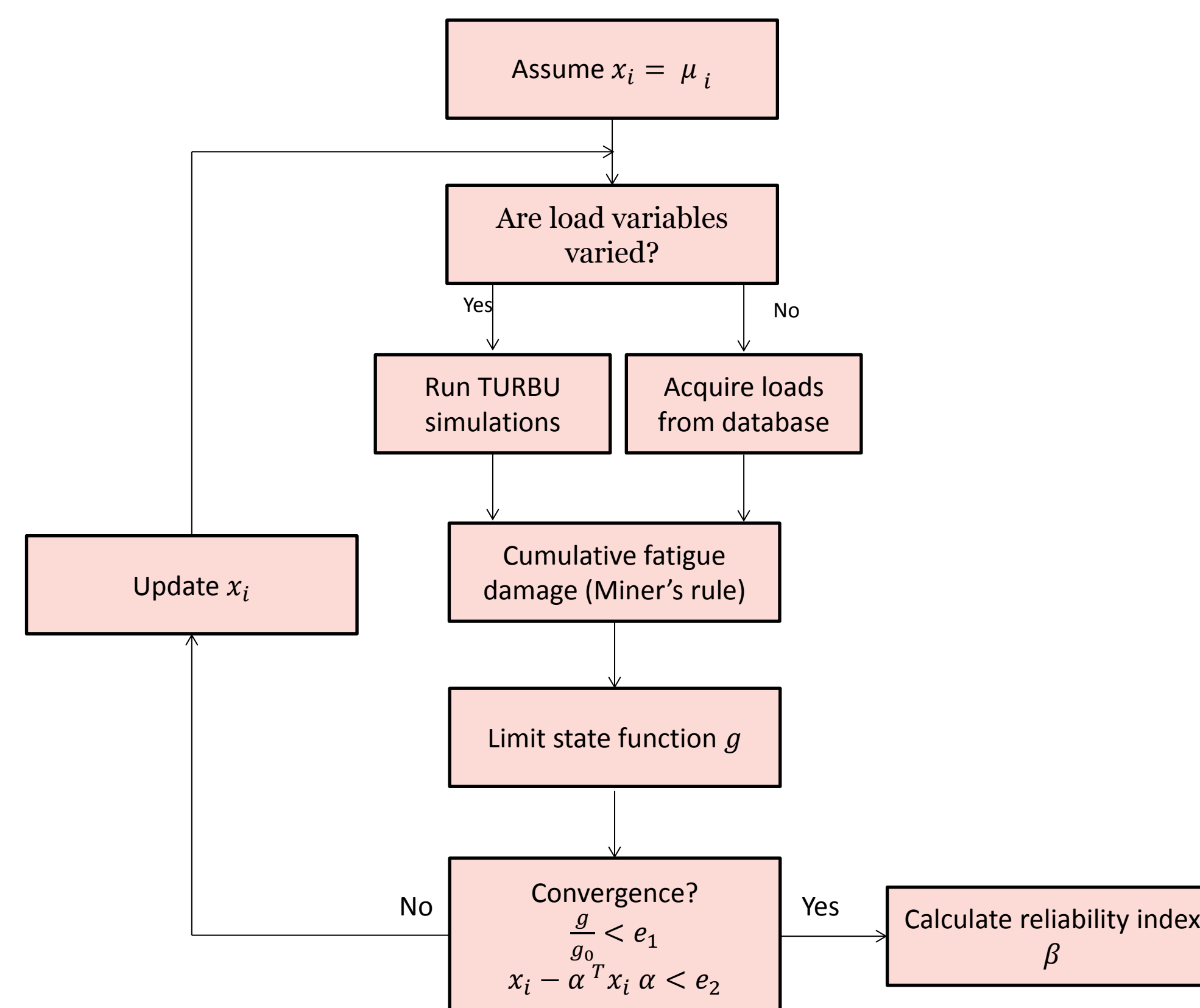
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Objective

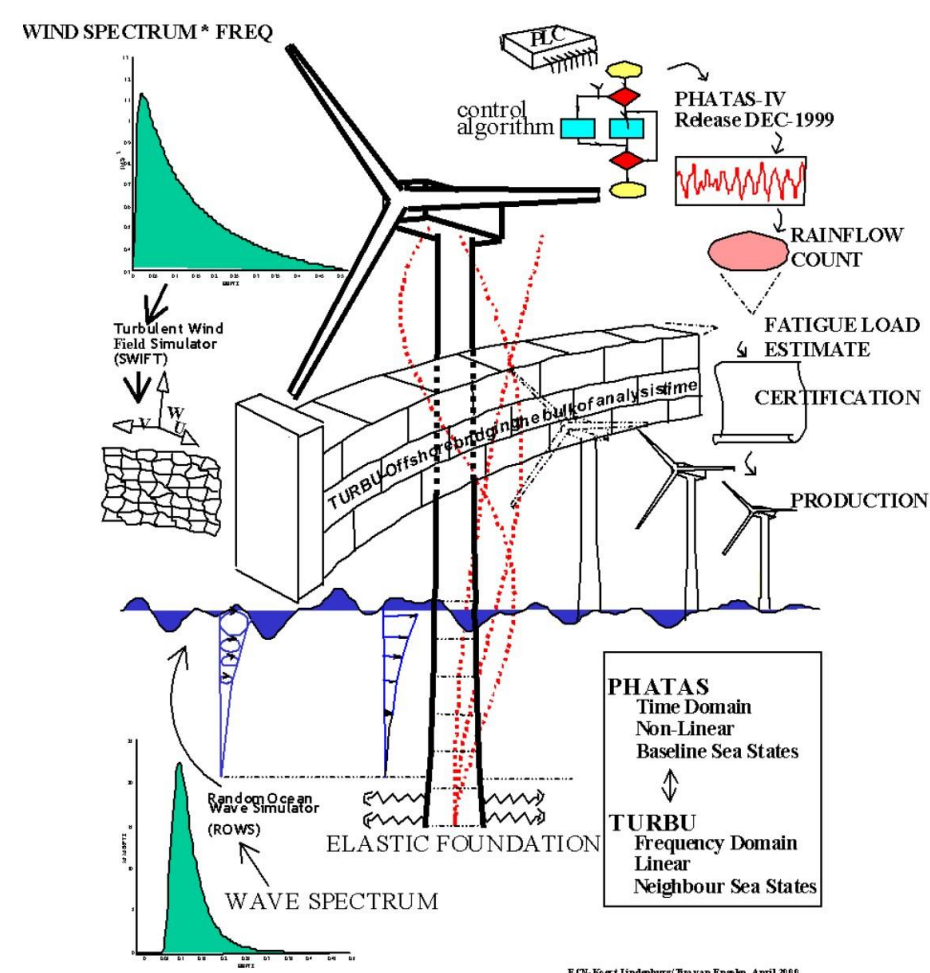
To investigate where cost reduction are possible in the support structure while keeping a sound and safe design:

- Probabilistic design methods are used.
- For time efficient load computations TURBU, a fast fully integrated wind turbine design and analysis tool in the frequency domain, is integrated in the probabilistic approach.



TURBU

- Full non-linear steady state model (multi-body average deformation)
- Time-invariant linear dynamic model (multi-body, Newton, Coleman)
- Linear frequency and time domain analysis of 3-bladed Horizontal Axis Wind turbines



Fatigue limit state:

$$g = \Delta - D = 0$$

$N_{max} = f(\log C1, \log C2)$ of SN-curve (DNV RP-C203)

$$D = \sum_i \frac{n_i}{N_{max,i}}$$

FERUM

- Open source structural reliability code in MATLAB.
- First Order Reliability Method (FORM) selected.
- Advantage FORM is information on contribution of selected stochastic variables to the variance of the limit state function g .

| Variable | Distribution | Mean | Standard deviation |
|------------------|--------------|----------|--------------------|
| logC1 | Normal | 12.164 | 0.20 |
| logC2 | Normal | 16.106 | 0.25 |
| Δ (Miner) | Lognormal | 1.00 | 0.30 |
| Young modulus | Lognormal | 210e9 | 42e9 |
| CD | Normal | 0.70 | 0.10 |
| CM | Normal | 2.00 | 0.10 |
| Soil stiffness | Lognormal | 6.603e10 | 1.321e10 |

Case study

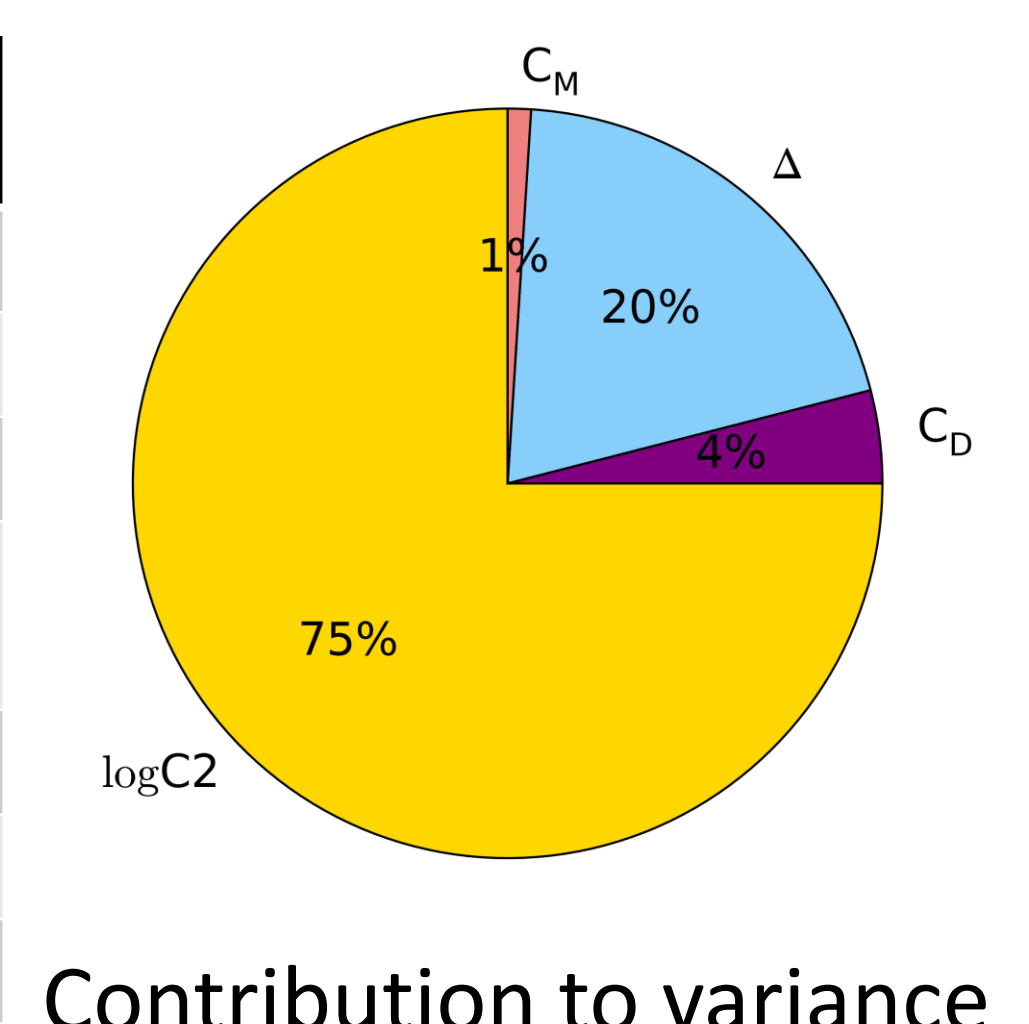
- Modern 4MW wind turbine with monopile support structure, rotor diameter 130m, in 30m water depth.
- Twelve wind bins with for every wind bin six time series of one hour.
- Windspeed Weibull distribution $k = 2.15$ and $u = 9.36$ m/s.

| Bin | Wind velocity [m/s] | Significant wave height [m] | Spectra Peak Period [s] | peak shape parameter (gamma) |
|-----|---------------------|-----------------------------|-------------------------|------------------------------|
| 1 | 3 | 0.375 | 4.5 | 1.00 |
| 2 | 5 | 0.625 | 4.5 | 1.00 |
| 3 | 7 | 0.875 | 4.5 | 1.24 |
| 4 | 9 | 1.125 | 5.5 | 1.00 |
| 5 | 11 | 1.375 | 5.5 | 1.43 |
| 6 | 13 | 1.875 | 6.5 | 1.34 |
| 7 | 15 | 2.375 | 7.5 | 1.17 |
| 8 | 17 | 3.125 | 7.5 | 2.39 |
| 9 | 19 | 3.875 | 8.5 | 2.19 |
| 10 | 21 | 4.375 | 9.5 | 1.69 |
| 11 | 23 | 5.125 | 9.5 | 2.52 |
| 12 | 25 | 6.375 | 10.5 | 2.63 |

Results

- Rainflow count of fore-aft bending moment at mudline only.
- Design reliability index $\beta > 3.7$ (DNV OS-J101)
- Reliability index $\beta = 6.35$ (Failure probability = $1E-10$) in case study.

| Variable | Design point | Contribution to variance limit state function |
|------------------|--------------|-----------------------------------------------|
| logC1 | 12.164 | 0% |
| logC2 | 14.72 | 75% |
| Δ (Miner) | 0.42 | 20% |
| Young modulus | 210e9 | 0% |
| C_D | 0.81 | 4% |
| C_M | 2.13 | 1% |
| Soil stiffness | 5.956e10 | 0% |



Conclusions and recommendations

- Integration of full load calculations in probabilistic design method (FORM) is successful for fatigue limit state at mudline.
- The contribution of the Miner rule (Delta) and SN-curve (logC2) variables to the variance of the limit state function is largest.
- Calculated reliability index $\beta = 6.35$ shows there is room for design optimisation.
- Ultimate limit state and additional locations still need to be included.

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